

URS

Lambeth Highways SuDS

Rain Garden Modelling
Report - Final

April 2013

47062094

Prepared for:
Lambeth Council

UNITED
KINGDOM &
IRELAND



REVISION SCHEDULE					
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1 INTRODUCTION

1.1 Background

URS was commissioned by Lambeth Council to assess the flood alleviation benefits of ‘Rain Gardens’ proposed along Chatsworth Way and Ardlui Road in Lambeth.

This package of works forms part of the wider Lambeth Highways SuDS project, being undertaken by a consortium of companies, as outlined below:

- Design of Rain Garden Scheme – undertaken by The Environmental Protection Group Ltd. (EPG);
- Modelling of SuDS Scheme – undertaken by URS;
- Public Engagement / Awareness – undertaken by Sustrans;
- Project Support and production of a Case Study Report for the project - undertaken by URS.

1.2 Rain Gardens

In its simplest form, a Rain Garden is a shallow depression, with absorbent, yet free draining soil and planted with vegetation that can withstand occasional temporary flooding. Rain gardens are designed to mimic the natural water retention of undeveloped land and to reduce the volume of rainwater running off into drains from impervious areas and treat low level pollution.¹

For the Lambeth Highways SuDS scheme, rain gardens, as kerb build outs (Figure 1.1), are proposed upstream of existing road gullies in Chatsworth Way and Ardlui Road in Lambeth. The Rain Gardens will intercept surface water runoff at or close to source; and allow storage, filtration, infiltration and evapotranspiration (evaporation from soil and from vegetation) so that the amount of runoff discharging to the sewer network is greatly reduced.



Source: *Rain Garden Guide*¹

Figure 1.1: Example Highway Rain Garden in Portland Oregon, United States

¹ Bob Bray, Dusty Gedge, Gary Grant & Lani Leuthvilay, 2012, Rain Garden Guide. www.raingardens.info

1.3 Rain Garden Modelling Study

This modelling study assesses the flood alleviation benefits of selected Rain Gardens along Chatsworth Way and Ardlui Road (Figure 1.2) during the 2 year, 30 year and 100 year critical duration storm events. The assessment is based on a comparison of maximum outflow and discharge volume to existing gullies, pre- and post-implementation of the proposed Rain Gardens.

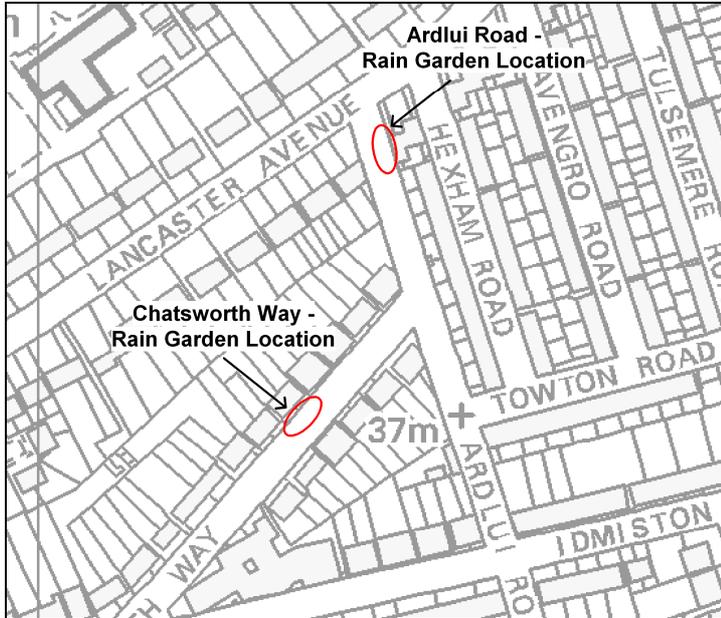


Figure 1.2: Modelled Rain Garden Locations for Lambeth Highways SuDS Scheme

Computer models of two of the proposed Rain Gardens have been built in Micro Drainage WinDes, based on typical layout and section drawings provided by Environmental Protection Group (EPG) (see Appendix A). A topographic survey with all of the potential Rain Garden locations is provided in Appendix B.

The modelling carried out compares the 'worst case' slope gradient (i.e. the steepest) Rain Garden along Ardlui Road with the 'best case' slope gradient (i.e. the most level) Rain Garden with a similar drainage catchment area along Chatsworth Way. This approach has been adopted to assess how Rain Gardens will function in 'best' and 'worst case' scenarios when considering the topography across the study area (see Figure 1.2).

In order to provide an indication of how sensitive the design / model is to different infiltration rates, a range of scenarios, ranging from low infiltration to high infiltration, has been considered.

2 MODELLING APPROACH

2.1 Overview

The WinDes DrawNet and Simulation modules have been used to build the two individual Rain Garden models and assess the impact of the proposed Rain Gardens in terms of rainwater source control and potential flood alleviation.

The existing baseline and Rain Garden scenarios have been tested against a number of different storm durations for the 2 year, 30 year and 100 year storm events. The storm duration results have been analysed to identify the critical 'worst case' storm when considering maximum discharge rate (l/s) and maximum discharge volume (m³).

2.2 Existing Scenario

To represent the existing baseline (no Rain Garden) scenario, sections of the existing Thames Water sewer network, including road gullies, have been modelled immediately upstream and downstream of the two selected Rain Gardens, one along Chatsworth Way and the other along Ardlui Road.

2.3 Rain Garden Scenario

The key Rain Garden design parameters (see Appendix A for typical layout and sections) represented in the model are:

- Depth of surface storage - 150 mm;
- Depth of Root Zone – 150 mm;
- Depth of Geocellular storage – 150 mm;
- Check dams – ~ 25 mm below existing kerb top level.

The two Rain Gardens have been represented using a series of surface storage units (tank / ponds) and below ground cellular storage, connected by a nominal pipe network (typically 100 mm to 150 mm in diameter), branching off from the existing gully. Upstream pipes represent each Rain Garden kerb inlet and have been assigned an appropriate catchment area within DrawNet, based on review of the topographic survey.

A series of online controls (weirs) have been used to represent check dams, which control surface water levels within the separate compartments. A small orifice (0.02 m diameter) has been used to represent the infiltration rate of surface water from the surface storage sections down into the cellular storage layer.

Once all of the surface storage units have been filled, the weir at the downstream end of the Rain Garden is overtopped and any further surface water finally drains to the Thames Water sewer, via the existing gully pot.

WinDes outputs, including a plan view of the individual Rain Garden pipe network and tabular details of the network, storage structures and online controls, are provided in Appendix C.

A summary of the key model parameters used to represent the individual Rain Gardens along Chatsworth Way and Ardlui Road are provided in the following sub-sections.

2.4 Rain Garden Model Parameters

Tables 2.1 and 2.2 provide the parameters used for the Chatsworth Way and Ardlui Road Rain Garden models.

TABLE 2.1: CHATSWORTH WAY RAIN GARDEN MODEL PARAMETERS			
Longitudinal fall (m)	0.1	Number of bays	1
Gully catchment area (m ²)	130	Diameter of outfall pipe to sewer (mm)	150
Below ground storage (m ³) (WinDes cellular storage)		Surface storage volume (m ³) (WinDes tank/pond)	
Structure 1	1.74	Structure 2	1.34
Total	1.74	Total	1.34

TABLE 2.2: ARDLUI ROAD RAIN GARDEN MODEL PARAMETERS			
Longitudinal fall (m)	0.48	Number of bays	3
Gully catchment area (m ²)	180	Diameter of outfall pipe to sewer (mm)	150
Below ground storage (m ³) (WinDes cellular storage)		Surface storage volume (m ³) (WinDes pond)	
Structure 1	0.62	Structure 2	0.54
Structure 3	0.86	Structure 4	0.57
Structure 5	0.23	Structure 6	0.23
Total Volume	1.71	Total Volume	1.34

2.5 Model Runs

For both of the selected locations, the following scenarios have been modelled:

- Existing baseline (no Rain Garden);
- Rain Garden (low infiltration – 0.001 m/hr);
- Rain Garden (medium infiltration – 0.01 m/hr);
- Rain Garden (high infiltration – 0.1 m/hr).

2.6 Assumptions and Limitations

The following assumptions and limitations relate to the modelled Rain Gardens:

- A limitation of the study is the selected Rain Gardens (based on 'worst case' and 'best case' slope gradients) have below average² gully catchment areas compared to other potential Rain Garden locations within the study area (see Appendix B). Rain Gardens with larger gully catchments receive greater volumes of runoff and therefore the percentage reduction in discharge volume entering the sewer is likely to be less;
- A limitation of modelling each Rain Garden in isolation is that WinDes assumes that when the volume of water exceeds the capacity of the Rain Garden, any flood water is stored locally on the surface and then fed back into the system as soon as capacity becomes available. However, in a 'real life' scenario excess water is likely to continue to flow down the road, entering a downstream Rain Garden or gully;
- The model assumes that all surface water generated within the gully catchment drains to the Rain Garden via the kerb inlets (i.e. no runoff by-passes the Rain Garden). However, in a 'real life' scenario some water is likely to by-pass the Rain Garden inlets, however this loss is unlikely to affect the results significantly;
- A standard gully pipe diameter of 150 mm has been assumed for the existing downstream gully pipe connecting the Rain Garden to the Thames Water sewer.

² The average gully catchment area for Rain Gardens within the study area is approximately 250 m² (gully catchment areas range from 100 m² to 500 m²)

3 RESULTS

3.1 Overview

The simulated maximum discharge rates and volumes during the 2 year, 30 year and 100 year critical duration storms have been taken at the existing downstream gully pipe, where the Rain Garden connects back into the sewer network. A comparison of results at this downstream location has been used to indicate the benefits provided by the Rain Gardens.

Tabulated results for the Chatsworth Way and the Ardlui Road Rain Garden are provided in Tables 3.1 to 3.3 and Tables 3.4 to 3.6 respectively. The gully pipes status, as indicated in the bullet points below, is also provided:

- 'ok' – when the maximum water level is lower than the pipe's soffit;
- 'surcharged' – when the maximum water level is above the pipe's soffit;
- 'flood' - when the maximum water level is above the upstream manhole cover (i.e. the capacity of the Rain Garden is exceeded).

The storm durations included in the assessment have been limited to the 24 hour storm duration (1440 min). Storm durations greater than this are generally more intermittent and/or less intense in nature, and not suitable for consideration at the gully catchment scale.

Graphs have been produced for the critical storm duration, comparing downstream hydrographs for the existing gully pipe and Rain Garden scenario. The medium infiltration Rain Garden has been selected to illustrate the benefits of the scheme.

3.2 Chatsworth Way

This section presents the results of modelling the Chatsworth Way Rain Garden.

3.2.1 2 Year Critical Storm

Table 3.1 and Figures 3.1(a) and 3.1(b) present the results of modelling the Chatsworth Way Rain Garden for the 2 year critical storm.

Table 3.1 and Figure 3.1 (a) indicate that negligible flow discharges to the sewer via the downstream gully pipe occurs during the maximum outflow 2 year event once the Chatsworth Way Rain Garden is implemented.

Table 3.1 indicates that during the maximum discharge volume 2 year event the Rain Garden scenario reduces the volume discharged to the sewer by a minimum of 61% (low infiltration scenario). During the medium and high infiltration Rain Garden scenario the discharge volume to the sewer is reduced by 75% and 88% respectively with negligible discharge to the downstream sewer network (Figure 3.1 (b)).

TABLE 3.1: CHATSWORTH WAY - 2 YEAR CRITICAL STORM								
Rain Garden (RG) description	Maximum discharge rates				Maximum discharge volumes			
	Max rate (l/s)	Critical storm duration (min)	Pipe Status	Discharge rate reduction (%)	Maximum discharge volume (m ³)	Critical storm duration (min)	Pipe Status	Discharge volume reduction (%)
Existing gully	2.6	15 Summer	ok	NA	4.3	1440 Winter	ok	NA
RG - low infiltration (0.001 m/hr)	0.1	1440 Summer	ok	96	1.7	1440 Winter	ok	61
RG - medium infiltration (0.01 m/hr)	0.1	120 Summer	ok	96	1.1	1440 Summer	ok	75
RG - high infiltration (0.1 m/hr)	0.1	120 Summer	ok	96	0.5	1440 Winter	ok	88

Figure 3.1 (a) Chatsworth Way - 2 year (120 min summer storm)

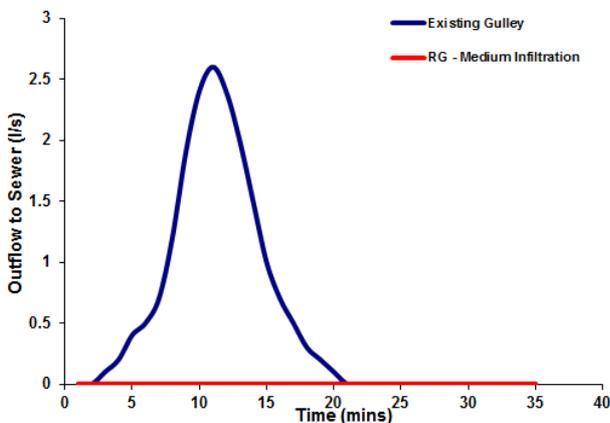
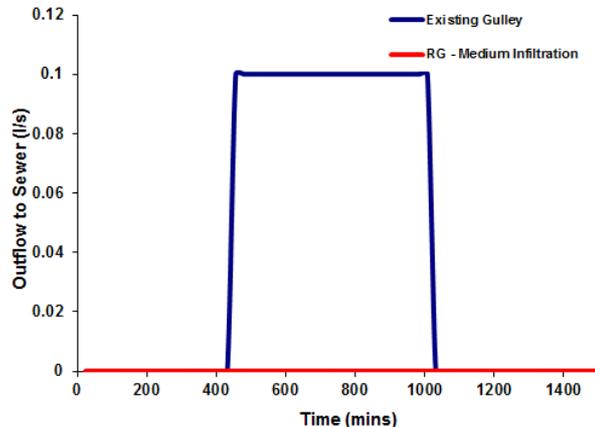


Figure 3.1 (b) Chatsworth Way - 2 year (1440 min summer storm)



3.2.2 30 Year Critical Storm

Table 3.2 and Figures 3.2(a) and 3.2(b) present the results of modelling the Chatsworth Way Rain Garden for the 30 year critical storm.

Table 3.2 and Figure 3.2 (a) indicates that maximum discharge to the sewer is limited to 3.9 l/s for the maximum outflow 30 year event Rain Garden scenarios. Figure 3.2 (a) indicates that no discharge to the sewer occurs until 10 minutes.

Table 3.2 indicates that during the maximum discharge volume 30 year event the Rain Garden scenario reduces the volume discharged to the sewer by a minimum of 34% (low infiltration scenario). During the medium to high infiltration Rain Garden scenario the discharge volume to the sewer is reduced by 47% and 85% respectively.

Figure 3.2 (b) indicates how the Rain Garden attenuates runoff for the first 700 minutes of the storm, after which the Rain Garden capacity is exceeded and runoff discharges to the sewer.

TABLE 3.2: CHATSWORTH WAY - 30 YEAR CRITICAL STORM

Rain Garden (RG) description	Maximum discharge rates				Maximum discharge volumes			
	Max rate (l/s)	Critical storm duration (min)	Pipe Status	Discharge rate reduction (%)	Maximum discharge volume (m ³)	Critical storm duration (min)	Pipe Status	Discharge volume reduction (%)
Existing gully	6.4	15 Summer	ok	NA	8.9	1440 Summer	ok	NA
RG - low infiltration (0.001 m/hr)	3.9	15 Summer	ok	39	5.9	1440 Summer	ok	34
RG - medium infiltration (0.01 m/hr)	3.9	15 Summer	ok	39	4.7	1440 Summer	ok	47
RG - high infiltration (0.1 m/hr)	3.9	15 Summer	ok	39	1.3	360 Summer	ok	85

Figure 3.2 (a) Chatsworth Way - 30 year (15 min summer storm)

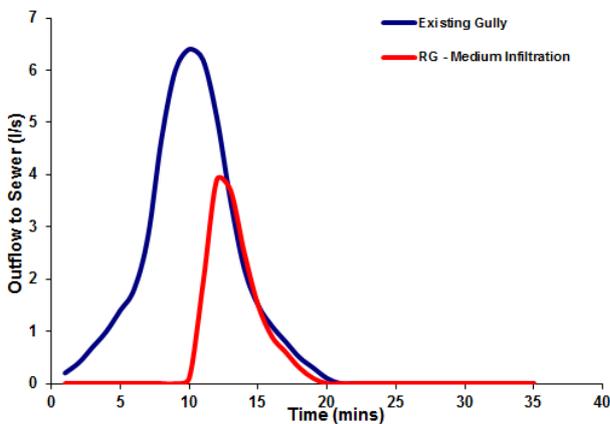
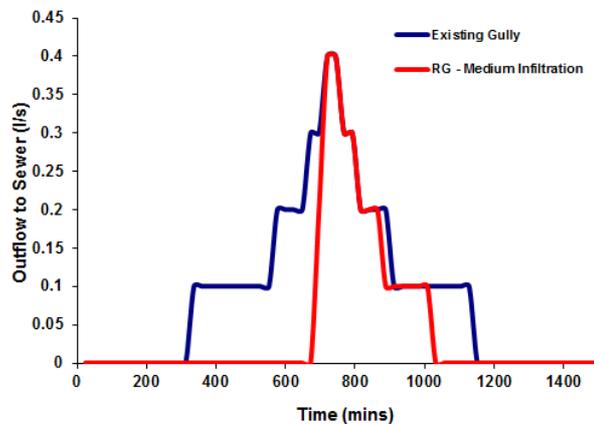


Figure 3.2 (b) Chatsworth Way - 30 year (1440 min summer storm)



3.2.3 100 Year Critical Storm

Table 3.3 and Figures 3.3(a) and 3.3(b) present the results of modelling the Chatsworth Way Rain Garden for the 100 year critical storm.

Table 3.3 indicates that maximum discharge to the sewer is limited to 8.1 l/s for the 100 year event Rain Garden scenarios (16% reduction compared to baseline). Figure 3.3 (a) indicates that no discharge to the sewer occurs until 8 minutes.

Table 3.3 indicates that during the maximum discharge volume 100 year event the Rain Garden scenario reduces the volume discharged to the sewer by a minimum of 26% (low infiltration scenario). During the medium to high infiltration Rain Garden scenario the discharge volume to the sewer is reduced by 36% and 73% respectively. Figure 3.3 (b) indicates how the Rain Garden attenuates runoff until its capacity is reached after approximately 600 minutes.

TABLE 3.3: CHATSWORTH WAY - 100 YEAR CRITICAL STORM								
Rain Garden (RG) description	Maximum discharge rates				Maximum discharge volumes			
	Max rate (l/s)	Critical storm duration (min)	Pipe Status	Discharge rate reduction (%)	Maximum discharge volume (m ³)	Critical storm duration (min)	Pipe Status	Discharge volume reduction (%)
Existing gully	9.7	15 Summer	ok	NA	11.9	1440 Summer	ok	NA
RG - low infiltration (0.001 m/hr)	8.1	15 Summer	ok	16	8.8	1440 Summer	ok	26
RG - medium infiltration (0.01 m/hr)	8.1	15 Summer	ok	16	7.6	1440 Summer	ok	36
RG - high infiltration (0.1 m/hr)	8.1	15 Summer	ok	16	3.3	480 Summer	ok	73

Figure 3.3 (a) Chatsworth Way - 100 year (15 min summer storm)

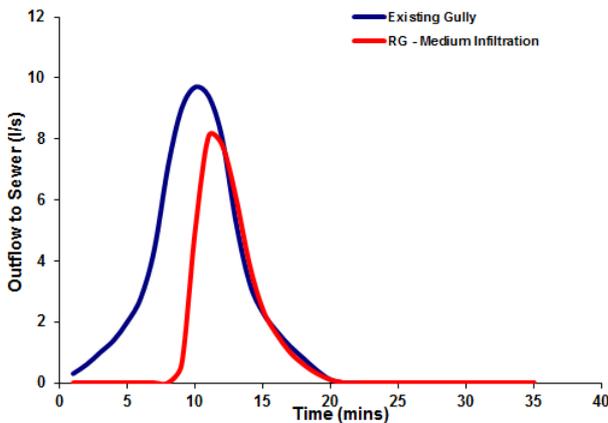
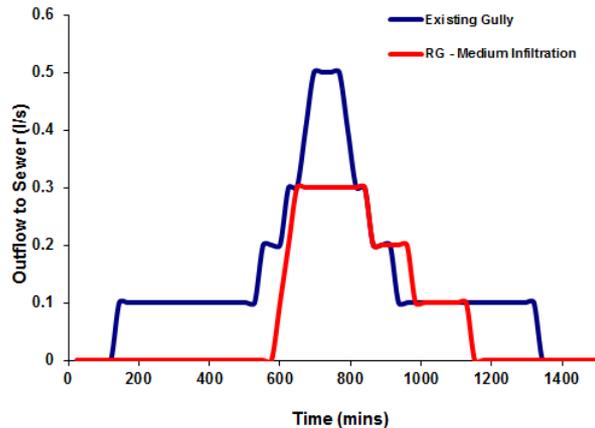


Figure 3.3 (b) Chatsworth Way - 100 year (1440 min summer storm)



3.3 Ardlui Road

This section presents the results of modelling the Ardlui Road Rain Garden.

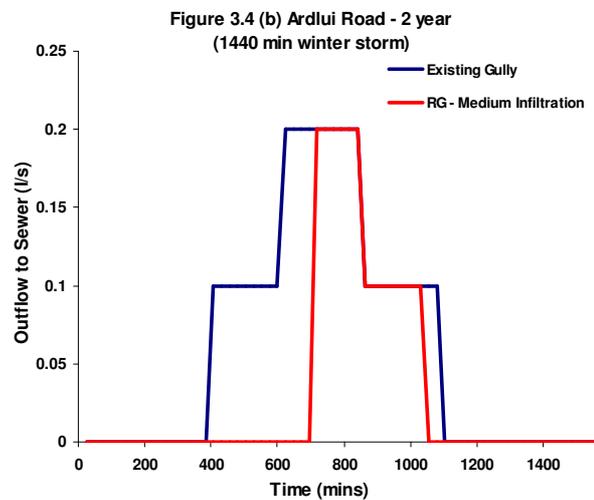
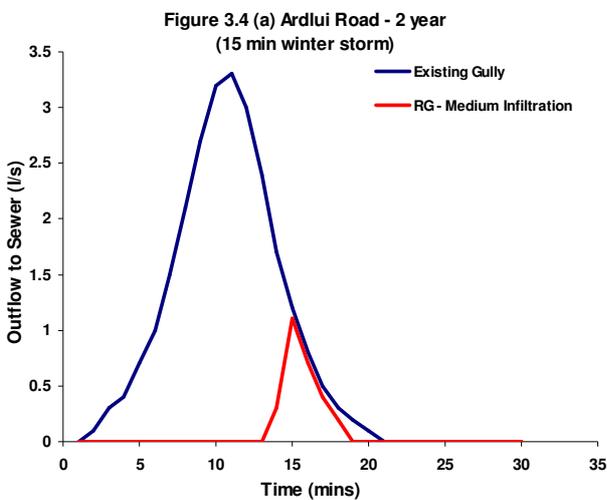
3.3.1 2 Year Critical Storm

Table 3.4 and Figures 3.4(a) and 3.4(b) present the results of modelling the Ardlui Road Rain Garden for the 2 year critical storm.

Table 3.4 indicates that maximum discharge to the sewer is limited to 1.0 l/s during the 2 year event Rain Garden scenarios. Figure 3.4 (a) indicates that no discharge to the sewer occurs until 14 minutes.

Table 3.4 indicates that during the maximum discharge volume 2 year event the Rain Garden scenario reduces the volume discharged to the sewer by a minimum of 34% (low infiltration scenario). During the medium to high infiltration Rain Garden scenario the discharge volume to the sewer is reduced by 54% and 100% respectively. Figure 3.4 (b) indicates how the Rain Garden attenuates runoff until its capacity is reached after 700 minutes.

TABLE 3.4: ARDLUI ROAD - 2 YEAR CRITICAL STORM								
Rain Garden (RG) description	Maximum discharge rates				Maximum discharge volumes			
	Max rate (l/s)	Critical storm duration (min)	Pipe Status	Discharge rate reduction (%)	Maximum discharge volume (m3)	Critical storm duration (min)	Pipe Status	Discharge volume reduction (%)
Existing gully	3.3	15 Winter	ok	NA	6.1	1440 Winter	ok	NA
RG - low infiltration (0.001 m/hr)	1.0	15 Winter	ok	70	4.0	1440 Winter	ok	34
RG - medium infiltration (0.01 m/hr)	1.0	15 Winter	ok	70	2.8	1440 Winter	ok	54
RG - high infiltration (0.1 m/hr)	1.0	15 Winter	ok	70	0	1440 Winter	ok	100



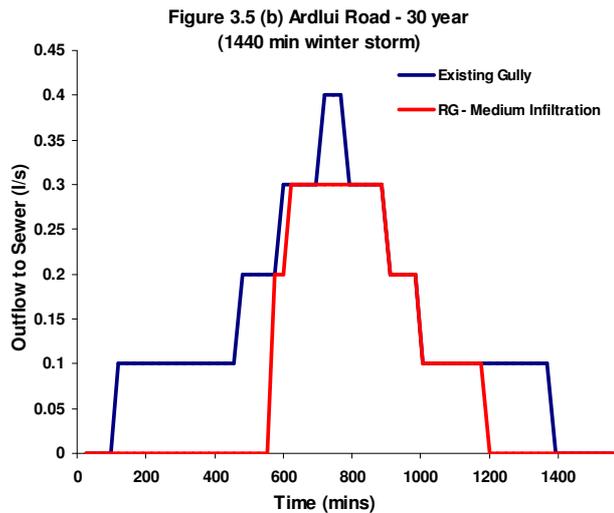
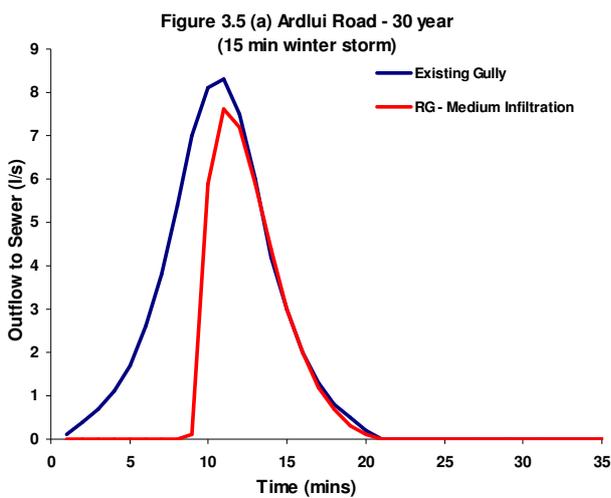
3.3.2 30 Year Critical Storm

Table 3.5 and Figures 3.5(a) and 3.5(b) present the results of modelling the Ardlui Road Rain Garden for the 30 year critical storm.

Table 3.5 indicates that maximum discharge to the sewer is limited to 7.6 l/s during the 30 year event Rain Garden scenarios. Figure 3.5 (a) indicates that no discharge to the sewer occurs until 9 minutes.

Table 3.5 indicates that during the maximum discharge volume 30 year event the Rain Garden scenario reduces the volume discharged to the sewer by a minimum of 20% (low infiltration scenario). During the medium to high infiltration Rain Garden scenario the discharge volume to the sewer is reduced by 29% and 89% respectively. Figure 3.5 (b) indicates how the Rain Garden attenuates runoff until its capacity is reached after 570 minutes.

TABLE 3.5: ARDLUI ROAD - 30 YEAR CRITICAL STORM								
Rain Garden (RG) description	Maximum discharge rates				Maximum discharge volumes			
	Max rate (l/s)	Critical storm duration (min)	Pipe Status	Discharge rate reduction (%)	Maximum discharge volume (m ³)	Critical storm duration (min)	Pipe Status	Discharge volume reduction (%)
Existing gully	8.3	15 Winter	ok	NA	12.3	1440 Winter	ok	NA
RG - low infiltration (0.001 m/hr)	7.6	15 Winter	Flood	8	9.9	1440 Winter	ok	20
RG - medium infiltration (0.01 m/hr)	7.6	15 Winter	Flood	8	8.7	1440 Winter	ok	29
RG - high infiltration (0.1 m/hr)	7.6	15 Winter	Flood	8	1.4	1440 Winter	ok	89



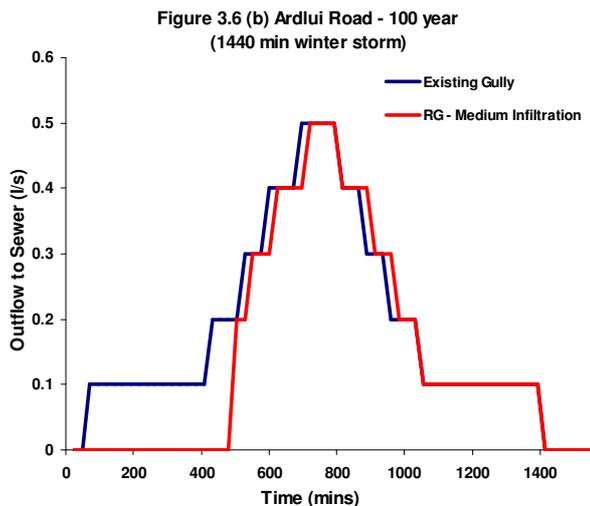
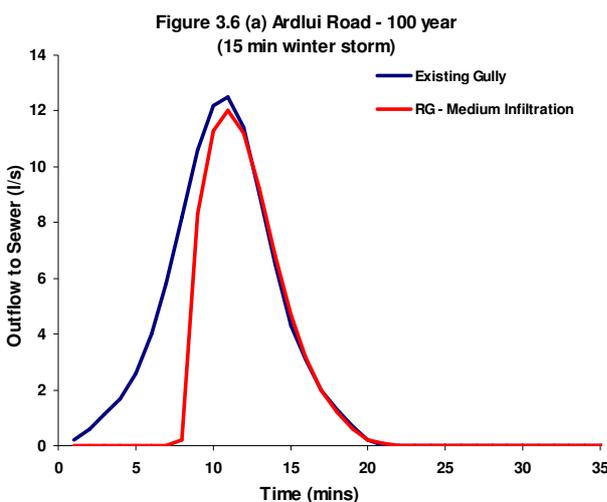
3.3.3 100 Year Critical Storm

Table 3.6 and Figures 3.6(a) and 3.6(b) present the results of modelling the Ardlui Road Rain Garden for the 100 year critical storm.

Table 3.6 indicates that maximum discharge to the sewer is limited to 12 l/s during the 100 year event Rain Garden scenarios, providing a 4% reduction compared to baseline. Figure 3.6 (a) indicates that no discharge to the sewer occurs until 8 minutes.

Table 3.6 indicates that during the maximum discharge volume 100 year event the Rain Garden scenario reduces the volume discharged to the sewer by a minimum of 13% (low infiltration scenario). During the medium to high infiltration Rain Garden scenario the discharge volume to the sewer is reduced by 22% and 74% respectively. Figure 3.6 (b) indicates how the Rain Garden attenuates runoff until its capacity is reached after 504 minutes.

TABLE 3.6: ARDLUI ROAD - 100 YEAR CRITICAL STORM								
Rain Garden (RG) description	Maximum discharge rates				Maximum discharge volumes			
	Max rate (l/s)	Critical storm duration (min)	Pipe Status	Discharge rate reduction (%)	Maximum discharge volume (m ³)	Critical storm duration (min)	Pipe Status	Discharge volume reduction (%)
Existing gully	12.5	15 Winter	ok	NA	16.4	1440 Winter	ok	NA
RG - low infiltration (0.001 m/hr)	12	15 Winter	Flood	4	14.2	1440 Winter	ok	13
RG - medium infiltration (0.01 m/hr)	12	15 Winter	Flood	4	12.8	1440 Winter	ok	22
RG - high infiltration (0.1 m/hr)	12	15 Winter	Flood	4	4.2	1440 Winter	ok	74



4 PREDICTOR TOOL

This study considers the performance of selected 'best case' and 'worst case' Rain Gardens where the case is based upon slope gradient alone. In reality, a number of other factors are likely to influence the final positioning of Rain Gardens within the study area.

To provide an indication of how Rain Gardens would perform at various proposed locations within the study area a simple predictor tool has been developed using Microsoft Excel.

The predictor tool is based on Flood Estimation Handbook (FEH) (CD-ROM3) rainfall depth, duration and frequency data extracted for the locality and assesses the performance of Rain Gardens based on the following design criteria input parameters:

- Rain Garden catchment area (m²);
- Surface storage volume (m³);
- Cellular storage volume (m³);
- Average infiltration (l/hr).

For each set of Rain Garden input parameters conditional formatting has been used to turn rainfall depth cells green where no discharge to sewer occurs and pink where some discharge to sewer occurs. The predictor tool also provides the following information for each Rain Garden scenario:

- Volume of runoff (m³);
- Volume of infiltration during event (m³);
- Volume of discharge to sewer (m³);
- Drain down time after event (hrs).

Chatsworth Way Rain Garden model parameters (see Table 2.1) have been used to demonstrate how the predictor tool can be applied. The predictor tool screen dump shown in Appendix D indicates that where considering the 2 year event no discharge to the sewer occurs up to the 315 minute duration storm. Appendix D indicates that the 2 year 315 minute storm requires a drain down time of approximately 43 hours.

It is important to note that this predictor tool only provides an indication of the Rain Garden performance and should only be used within the defined study area (Figure 1.2), as rainfall depth, duration and frequency data varies spatially. The tool should be used as a precursor to detailed modelling, of the type undertaken in WinDes for this study.

The current version of the predictor tool assumes rapid transfer from open storage to cellular storage, however a future version of the tool could be further developed to allow users to specify this parameter.

5 CONCLUSIONS

The conclusions of the Rain Garden modelling study are set out below:

- During more frequent, less severe storms (i.e. 2 year event), both the Ardlui Road (worst case) and Chatsworth Way (best case) gradient Rain Garden locations are shown to provide significant benefits in terms of reducing maximum rates and discharge volumes to the sewer;
- The reduction in maximum discharge rate, post Rain Garden implementation, is less for the Ardlui Road Rain Garden. This is likely due to the larger catchment draining to the Ardlui Road Rain Garden;
- The percentage reduction in maximum discharge volume, post Rain Garden implementation, is less for the Ardlui Road Garden during the low and medium infiltration scenarios when compared to the Chatsworth Way Rain Garden, but similar during the high infiltration scenarios;
- When higher cellular storage infiltration rates are modelled, the maximum discharge volumes at both the 'worst case' and 'best case' Rain Garden locations reduce;
- During less frequent 30 year and 100 year events, the shorter duration, higher intensity storms, associated with the maximum outflow critical storm, exceed the capacity of the 'worst case' Rain Garden. However, the capacity of the 'best case' Rain Garden is not exceeded. Again this is likely due to the smaller catchment draining to the Chatsworth Way Rain Garden;
- During the longer duration 30 year and 100 year events, associated with the maximum discharge critical storm, the capacity of both the 'worst case' and 'best case' Rain Gardens the pipe status remains 'ok';
- In summary, all cases modelled provide a degree of attenuation and storage which, cumulatively, would act to reduce surcharging pressure on the local drainage network.

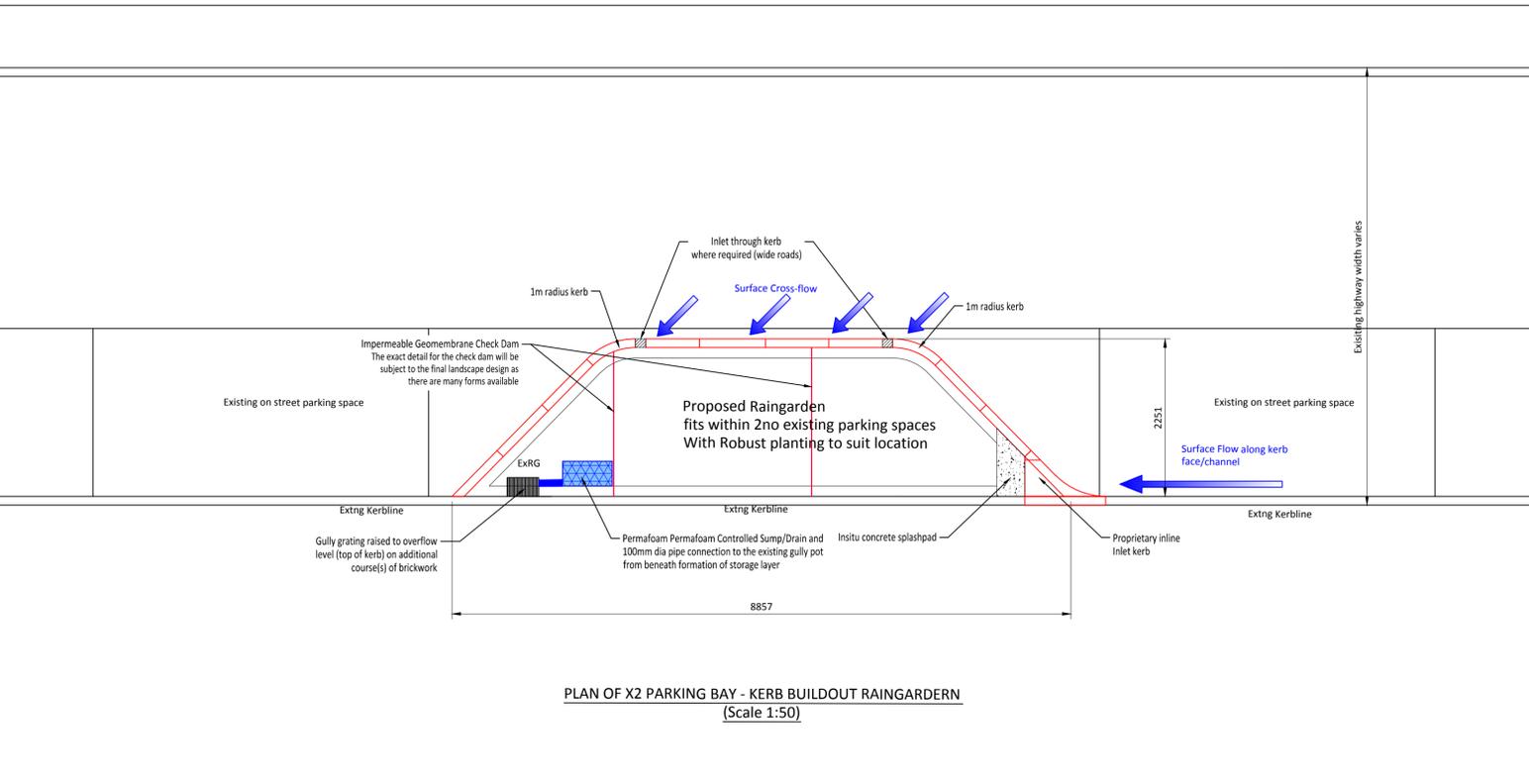
5.1 Further Work

To further inform and improve the modelling study the following recommendations are made:

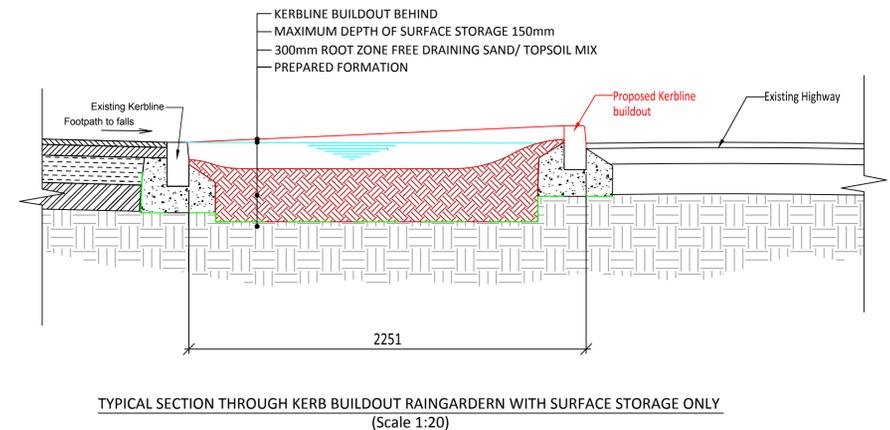
- Undertake WinDes modelling at further locations within the study area to assess the flood alleviation benefits of Rain Gardens within larger gully catchments;
- The interaction between a series of Rain Gardens (including the associated Thames Water sewer network) could be assessed using WinDes FloodFlow. FloodFlow includes the surface terrain allowing excess water exceeding the capacity of the Rain Garden (or bypasses the Rain Garden) to follow existing drainage pathways;
- The FloodFlow approach will allow the flooded volume to feed back into the system at a downstream point, rather than returning back into the system at the same point, as assumed in the current model. This would improve estimates of attenuation performance;
- To improve the understanding of existing flood risk and potentially provide further evidence for the requirement of Rain Gardens in the locality, a baseline model of the existing Thames Water sewer network could be built using WinDes (including FloodFlow). A review of the current Thames Water sewer network dataset held by URS has identified a number of missing pipe invert levels within the study area, this information would be required to inform the baseline model;

- Further development of the predictor tool to allow users to specify rate of transfer from open storage to cellular storage and to benchmark tool performance against model outputs such that in future the tool can be used with confidence to predict Rain Garden performance across larger areas, reducing the immediate need for modelling at the planning stage.

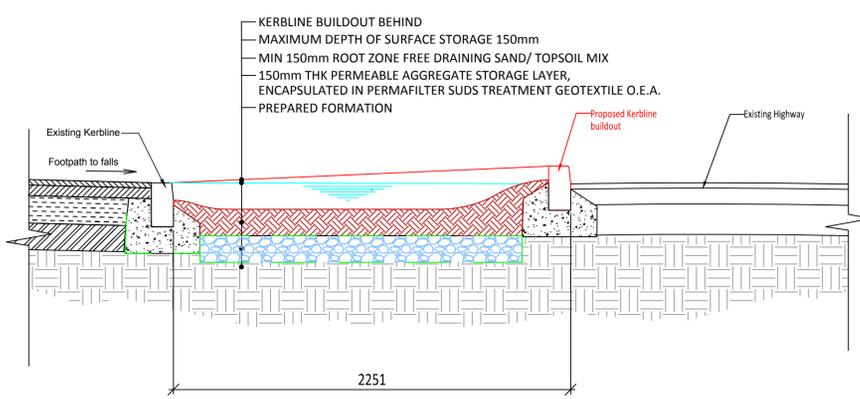
APPENDIX A – TYPICAL LAYOUT & SECTION



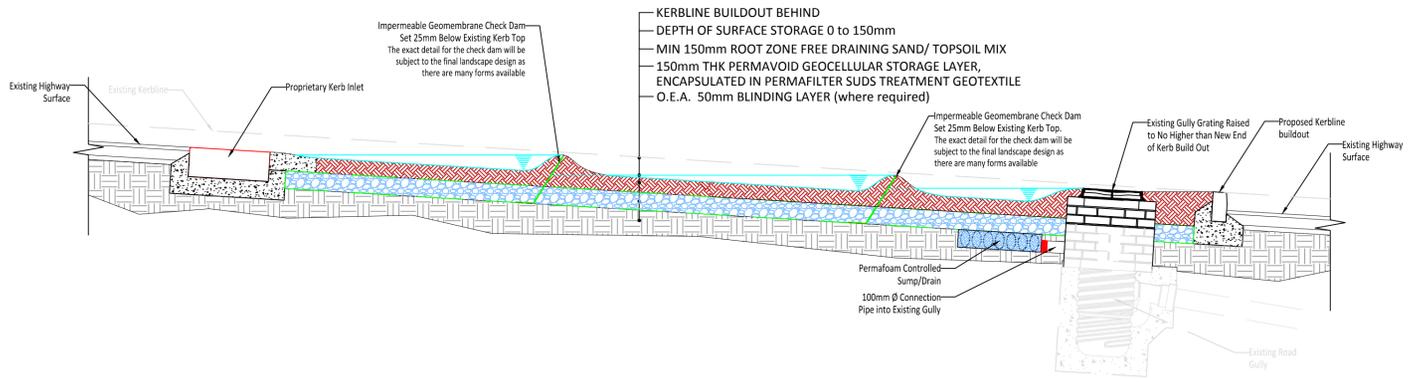
PLAN OF X2 PARKING BAY - KERB BUILDOUT RAINGARDERN
(Scale 1:50)



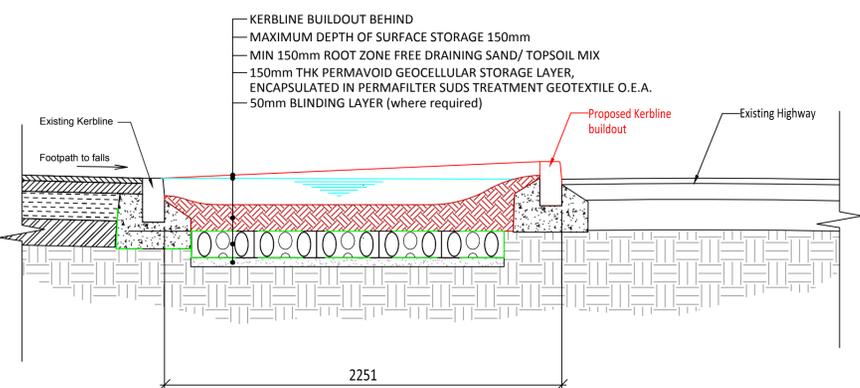
TYPICAL SECTION THROUGH KERB BUILDOUT RAINGARDERN WITH SURFACE STORAGE ONLY
(Scale 1:20)



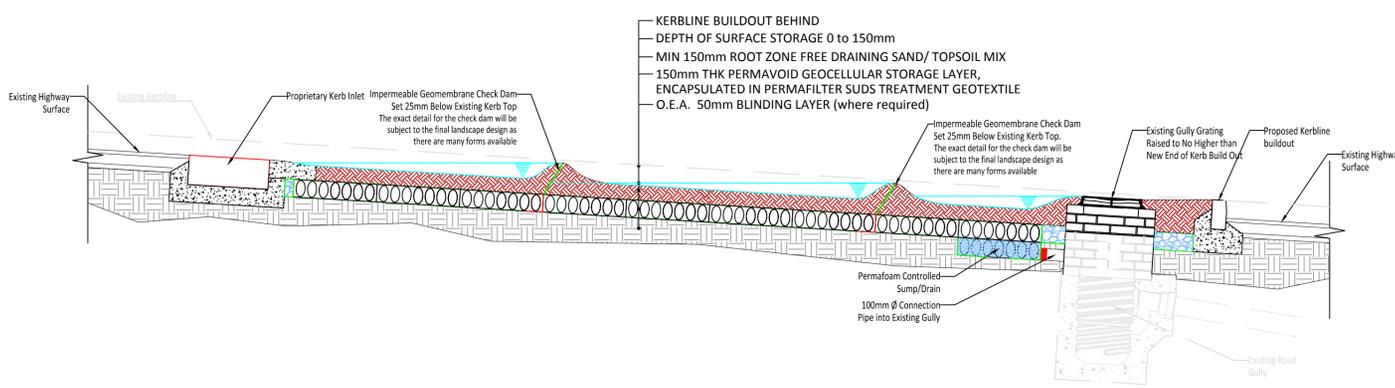
TYPICAL SECTION THROUGH KERB BUILDOUT RAINGARDERN WITH PERMEABLE AGGREGATE STORAGE
(Scale 1:20)



TYPICAL LONG SECTION THROUGH KERB BUILDOUT RAINGARDERN WITH PERMEABLE AGGREGATE STORAGE
(Scale 1:30)



TYPICAL SECTION THROUGH KERB BUILDOUT RAINGARDERN WITH GEOCELLULAR STORAGE
(Scale 1:20)



TYPICAL LONG SECTION THROUGH KERB BUILDOUT RAINGARDERN WITH GEOCELLULAR STORAGE
(Scale 1:30)

FOR COMMENT

REV	DESCRIPTION	DATE
P2	Checkdams & typical long-sections added	06/08/12
P1	Typical sections added	14/05/12



PROJECT
LAMBETH DIY STREETS

CLIENT
LONDON BOROUGH OF LAMBETH HIGHWAYS DEPARTMENT

DRAWING TITLE
SURFACE WATER MANAGEMENT KERB BUILD-OUT RAINGARDEN LAYOUT & SECTIONS

SCALE
As Shown @ A1

DATE
10/02/2012

DRAWING No
EPG/7754/SD/01

APPENDIX B – TOPOGRAPHIC SURVEY

APPENDIX C – WINDES OUTPUTS

Scott House
Alencon Link
Basingstoke RG21 7PP

Date 15/10/2012 09:41
File SW_high_Infiltra...

Designed by 34299mc
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Micro Drainage Network W.12.6

Existing Network Details for SW.txt

* - Indicates pipe has been modified outside of System 1

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	k (mm)	HYD SECT	DIA (mm)	
1.000	24.812	1.306	19.0	0.000	5.00	0.600	o	300	
* 2.000	1.000	0.005	200.0	0.000	5.00	0.600	o	100	
* 2.001	1.093	0.006	182.2	0.000	0.00	0.600	o	100	
3.000	3.413	0.200	17.1	0.013	5.00	0.600	o	150	
4.000	2.046	0.300	6.8	0.001	5.00	0.600	o	150	
* 2.002	2.802	0.150	18.7	0.002	0.00	0.600	o	150	
* 5.000	1.204	0.005	240.8	0.000	5.00	0.600	o	100	
* 5.001	1.499	0.006	249.8	0.000	0.00	0.600	o	100	
6.000	1.830	0.150	12.2	0.000	5.00	0.600	o	150	
* 2.003	1.000	0.060	16.7	0.002	0.00	0.600	o	150	
* 7.000	1.000	0.005	200.0	0.000	5.00	0.600	o	100	
* 7.001	1.000	0.006	166.7	0.000	0.00	0.600	o	100	
2.004	3.567	4.985	0.7	0.000	0.00	0.600	o	150	
PN	US/MH Name	US/CL (m)	US/IL (m)	US C.Depth (m)	DS/CL (m)	DS/IL (m)	DS C.Depth (m)	Ctrl	US/MH (mm)
1.000	1	40.370	35.210	4.860	39.400	33.904	5.196		1200
* 2.000	2	40.000	39.260	0.640	40.000	39.255	0.645		1
* 2.001	3	40.000	39.255	0.645	39.420	39.249	0.071	Orifice	1
3.000	2	39.600	39.449	0.001	39.420	39.249	0.021		1
4.000	3	39.700	39.549	0.001	39.420	39.249	0.021		1
* 2.002	4	39.420	39.249	0.021	39.270	39.099	0.021	Weir 1 x 2100	
* 5.000	7	40.000	39.110	0.790	40.000	39.105	0.795		1
* 5.001	8	40.000	39.105	0.795	39.270	39.099	0.071	Orifice	1
6.000	5	39.400	39.249	0.001	39.270	39.099	0.021		1
* 2.003	6	39.270	39.099	0.021	39.190	39.039	0.001	Weir 1 x 2100	
* 7.000	11	40.000	39.050	0.850	40.000	39.045	0.855		1
* 7.001	12	40.000	39.045	0.855	39.190	39.039	0.051	Orifice	1
2.004	7	39.190	39.039	0.001	39.400	34.054	5.196	Weir 1 x 1000	

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Existing Network Details for SW.txt

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	k (mm)	HYD SECT	DIA (mm)	
1.001	32.322	1.701	19.0	0.000	0.00	0.600	o	300	
8.000	4.219	5.088	0.8	0.000	5.00	0.600	o	150	
9.000	3.806	5.085	0.7	0.000	5.00	0.600	o	150	
1.002	34.883	1.836	19.0	0.000	0.00	0.600	o	300	
PN	US/MH Name	US/CL (m)	US/IL (m)	US C.Depth (m)	DS/CL (m)	DS/IL (m)	DS C.Depth (m)	Ctrl	US/MH (mm)
1.001	8	39.400	33.904	5.196	37.673	32.203	5.170		1
8.000	9	37.592	37.441	0.001	37.673	32.353	5.170		1
9.000	10	37.589	37.438	0.001	37.673	32.353	5.170		1
1.002	11	37.673	32.203	5.170	36.714	30.367	6.047		1

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Scott House Alencon Link Basingstoke RG21 7PP		
Date 15/10/2012 09:41 File SW_high_Infiltra...	Designed by 34299mc Checked by	
Micro Drainage	Network W.12.6	

Online Controls for SW.txt

Orifice Manhole: 3, DS/PN: 2.001, Volume (m³): 0.0

Diameter (m) 0.020 Discharge Coefficient 0.600 Invert Level (m) 39.255

Weir Manhole: 4, DS/PN: 2.002, Volume (m³): 0.1

Discharge Coef 0.544 Width (m) 2.100 Invert Level (m) 39.395

Orifice Manhole: 8, DS/PN: 5.001, Volume (m³): 0.0

Diameter (m) 0.020 Discharge Coefficient 0.600 Invert Level (m) 39.105

Weir Manhole: 6, DS/PN: 2.003, Volume (m³): 0.1

Discharge Coef 0.544 Width (m) 2.100 Invert Level (m) 39.245

Orifice Manhole: 12, DS/PN: 7.001, Volume (m³): 0.0

Diameter (m) 0.020 Discharge Coefficient 0.600 Invert Level (m) 39.045

Weir Manhole: 7, DS/PN: 2.004, Volume (m³): 0.0

Discharge Coef 0.544 Width (m) 1.000 Invert Level (m) 39.150

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 Basingstoke RG21 7PP

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Micro Drainage Network W.12.6

Storage Structures for SW.txt

Cellular Storage Manhole: 3, DS/PN: 2.001

Invert Level (m) 38.955 Safety Factor 2.0
 Infiltration Coefficient Base (m/hr) 0.10000 Porosity 0.95
 Infiltration Coefficient Side (m/hr) 0.10000

Depth (m)	Area (m ²)	Inf. Area (m ²)	Depth (m)	Area (m ²)	Inf. Area (m ²)
0.000	4.1	4.1	0.151	0.0	5.3
0.150	4.1	5.3			

Tank or Pond Manhole: 4, DS/PN: 2.002

Invert Level (m) 39.249

Depth (m)	Area (m ²)	Depth (m)	Area (m ²)	Depth (m)	Area (m ²)
0.000	0.0	0.100	3.3	0.150	4.1

Cellular Storage Manhole: 8, DS/PN: 5.001

Invert Level (m) 38.800 Safety Factor 2.0
 Infiltration Coefficient Base (m/hr) 0.10000 Porosity 0.95
 Infiltration Coefficient Side (m/hr) 0.10000

Depth (m)	Area (m ²)	Inf. Area (m ²)	Depth (m)	Area (m ²)	Inf. Area (m ²)
0.000	5.7	5.7	0.151	0.0	7.1
0.150	5.7	7.1			

Tank or Pond Manhole: 6, DS/PN: 2.003

Invert Level (m) 39.099

Depth (m)	Area (m ²)						
0.000	0.0	0.050	1.9	0.100	3.8	0.150	5.7

Cellular Storage Manhole: 12, DS/PN: 7.001

Invert Level (m) 38.745 Safety Factor 2.0
 Infiltration Coefficient Base (m/hr) 0.10000 Porosity 0.95
 Infiltration Coefficient Side (m/hr) 0.10000

Depth (m)	Area (m ²)	Inf. Area (m ²)	Depth (m)	Area (m ²)	Inf. Area (m ²)
0.000	1.5	1.5	0.151	0.0	2.2
0.150	1.5	2.2			

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Tank or Pond Manhole: 7, DS/PN: 2.004

Invert Level (m) 39.039

Depth (m)	Area (m ²)	Depth (m)	Area (m ²)
0.000	0.0	0.150	1.5

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Basingstoke RG21 7PP

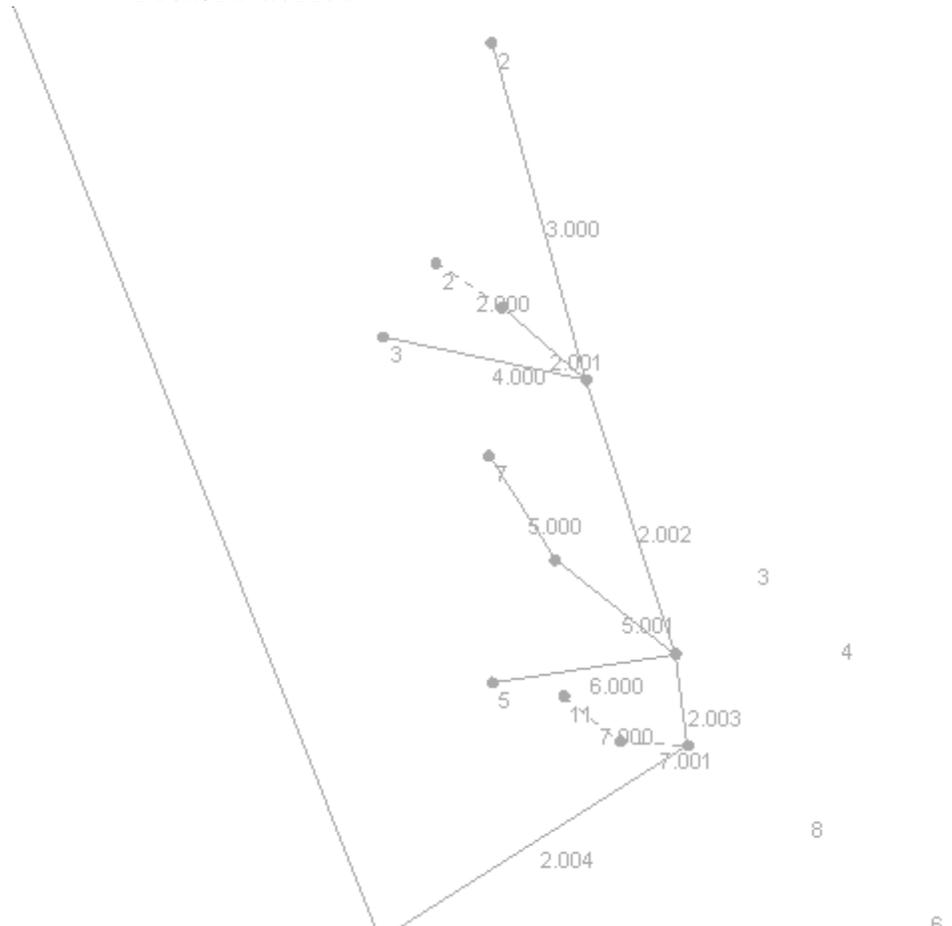


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Network W.12.6



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Existing Network Details for Storm

* - Indicates pipe has been modified outside of System 1

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	k (mm)	HYD SECT	DIA (mm)	
S1.000	16.566	0.240	69.0	0.000	5.00	0.600	o	300	
S2.000	4.168	0.100	41.7	0.000	5.00	0.600	o	150	
S3.000	4.041	0.140	28.9	0.000	5.00	0.600	o	150	
S1.001	19.775	0.160	123.6	0.000	0.00	0.600	o	300	
S4.000	7.144	0.100	71.4	0.009	5.00	0.600	o	150	
* S5.000	5.432	0.180	30.2	0.004	5.00	0.600	o	150	
S6.000	1.746	0.005	349.2	0.000	5.00	0.600	o	100	
S6.001	2.511	0.006	418.5	0.000	0.00	0.600	o	100	
S4.001	3.753	4.840	0.8	0.000	5.00	0.600	o	150	
* S7.000	4.019	4.710	0.9	0.000	5.00	0.600	o	150	
* S1.002	36.721	0.300	122.4	0.000	0.00	0.600	o	300	
* S8.000	3.798	4.690	0.8	0.000	5.00	0.600	o	150	
PN	US/MH Name	US/CL (m)	US/IL (m)	US C.Depth (m)	DS/CL (m)	DS/IL (m)	DS C.Depth (m)	Ctrl	US/MH (mm)
S1.000	1	37.470	32.470	4.700	37.220	32.230	4.690		1200
S2.000	2	37.220	32.220	4.850	37.220	32.120	4.950		1
S3.000	3	37.220	32.220	4.850	37.220	32.080	4.990		1
S1.001	2	37.220	32.220	4.700	37.060	32.060	4.700		1
S4.000	5	37.220	37.000	0.070	37.220	36.900	0.170		1
* S5.000	6	37.320	37.080	0.090	37.220	36.900	0.170		1
S6.000	7	37.220	36.911	0.209	37.220	36.906	0.214		1
S6.001	8	37.220	36.906	0.214	37.220	36.900	0.220	Orifice	1
S4.001	5	37.220	36.900	0.170	37.060	32.060	4.850	Weir 1 x 1000	
* S7.000	13	36.920	36.770	0.000	37.060	32.060	4.850		1
* S1.002	3	37.060	32.060	4.700	36.760	31.760	4.700		1
* S8.000	11	36.600	36.450	0.000	36.760	31.760	4.850		1

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Existing Network Details for Storm

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	k (mm)	HYD SECT	DIA (mm)	
* S9.000	3.765	4.690	0.8	0.000	5.00	0.600	o	150	
* S1.003	3.434	0.020	171.7	0.000	0.00	0.600	o	300	
PN	US/MH Name	US/CL (m)	US/IL (m)	US C.Depth (m)	DS/CL (m)	DS/IL (m)	DS C.Depth (m)	Ctrl	US/MH (mm)
* S9.000	12	36.600	36.450	0.000	36.760	31.760	4.850		1
* S1.003	4	36.760	31.760	4.700	36.740	31.740	4.700		1

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Scott House Alencon Link Basingstoke RG21 7PP		
Date 08/03/2013 09:35 File SW_F_Med_Infiltr...	Designed by 34299mc Checked by	
Micro Drainage	Network W.12.6	

Online Controls for Storm

Orifice Manhole: S8, DS/PN: S6.001, Volume (m³): 0.0

Diameter (m) 0.040 Discharge Coefficient 0.600 Invert Level (m) 36.906

Weir Manhole: S5, DS/PN: S4.001, Volume (m³): 0.2

Discharge Coef 0.544 Width (m) 1.000 Invert Level (m) 37.000

Scott House
 Alencon Link
 Basingstoke RG21 7PP



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Micro Drainage

Network W.12.6

Storage Structures for Storm

Cellular Storage Manhole: S8, DS/PN: S6.001

Invert Level (m) 36.600 Safety Factor 2.0
 Infiltration Coefficient Base (m/hr) 0.01000 Porosity 0.95
 Infiltration Coefficient Side (m/hr) 0.01000

Depth (m)	Area (m ²)	Inf. Area (m ²)	Depth (m)	Area (m ²)	Inf. Area (m ²)
0.000	11.6	11.6	0.151	0.0	15.0
0.150	11.6	15.0			

Tank or Pond Manhole: S5, DS/PN: S4.001

Invert Level (m) 36.900

Depth (m)	Area (m ²)	Depth (m)	Area (m ²)
0.000	11.6	0.113	11.6

Scott House
Alencon Link
Basingstoke RG21 7PP

Date 08/03/2013 09:39

File SW_F_Med_Infiltration_75%_TEST...

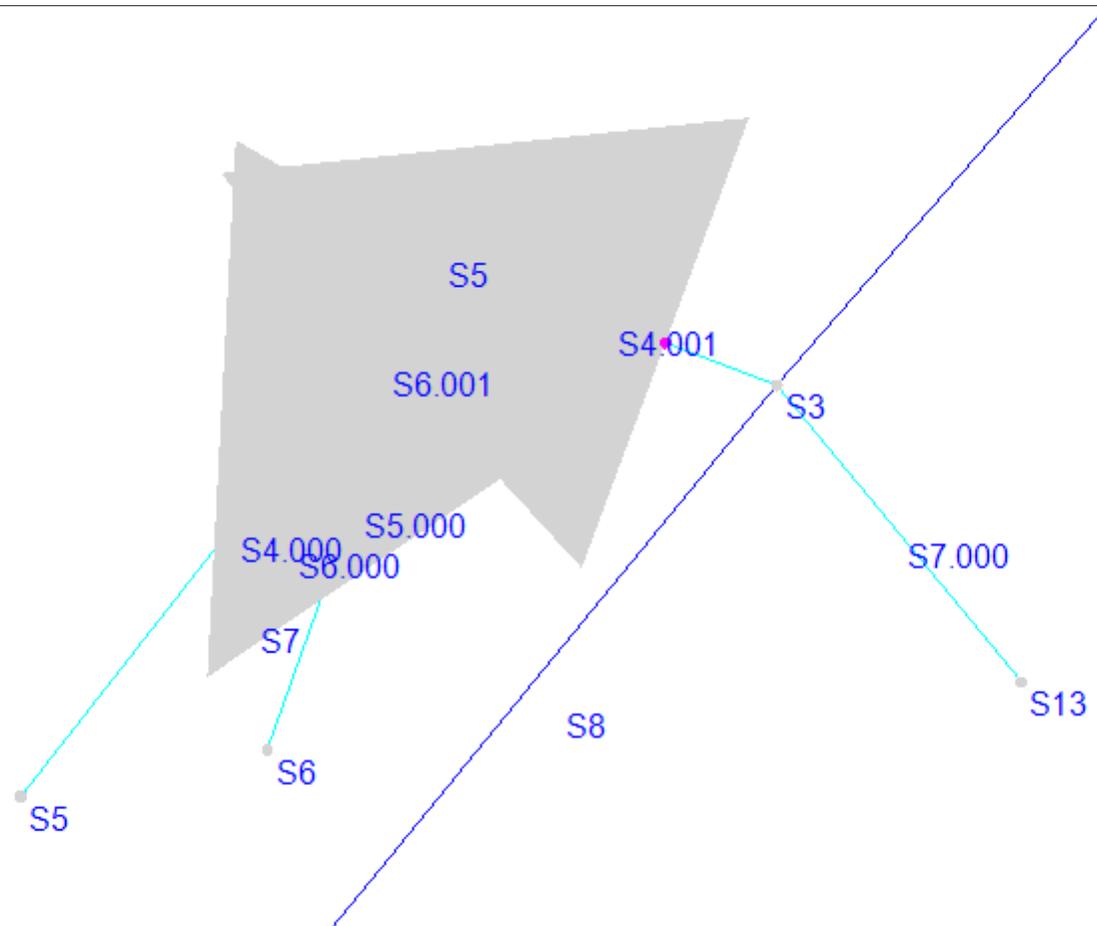
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Micro Drainage

Network W.12.6



APPENDIX D – LAMBETH RAIN GARDEN PREDICTOR TOOL

Predictor Tool example, based on the Chatsworth Way Rain Garden. The URS Lambeth Rain Garden Predictor Tool v2.0 (47062094 - URS Lambeth Rain Garden Predictor Tool_v2.0.xls) has been provided electronically alongside this report.

URS
Lambeth Council – Rain Garden Predictor Tool PLEASE SEE NOTES TAB
 Spreadsheet version 2.0
 Only valid for use within the Ardlui Road / Chatsworth Way area of the London Borough of Lambeth (should not be used to inform detailed design)

Rain Garden total impermeable catchment area (m²) 130 surface storage volume (m³) 1.34 cellular storage volume (m³) 1.74 average infiltration (litres / hour) 72

Selected depth (mm) 26.49 Volume of runoff (m³) 3.4437 Volume of discharge to sewer (m³) 0
 elected duration (hr) 5.25 Volume of infiltration during event (m³) 0.378 Drain down time after event (hours) 42.6

Duration hours	Duration minutes	0.08		0.17		0.25		0.33		0.42		0.5		0.58		0.67		0.75		0.83		0.92		1		1.5		2		5		10		20		50		100		200		500	
		1 month	2 months	3 months	4 months	5 months	6 months	7 months	8 months	9 months	10 months	11 months	12 months	18 months	2 years	5 years	10 years	20 years	50 years	100 years	200 years	500 years																					
0.25	15	3.46	4.39	5.04	5.56	6	6.39	6.73	7.04	7.33	7.6	7.85	8.09	9.29	10.24	14	17.73	22.45	30.69	38.87	49.23	67.27																					
0.5	30	4.52	5.68	6.47	7.1	7.64	8.1	8.52	8.9	9.25	9.57	9.87	10.15	11.58	12.72	17.13	21.45	26.87	36.19	45.33	56.79	76.48																					
0.75	45	5.29	6.59	7.49	8.2	8.8	9.32	9.78	10.2	10.59	10.95	11.28	11.6	13.18	14.43	19.27	23.98	29.85	39.86	49.61	61.74	82.44																					
1	60	5.91	7.33	8.3	9.07	9.73	10.29	10.79	11.24	11.66	12.04	12.41	12.74	14.44	15.79	20.95	25.96	32.16	42.68	52.88	65.51	86.95																					
1.25	75	6.44	7.96	9	9.82	10.51	11.11	11.64	12.12	12.56	12.97	13.35	13.71	15.51	16.93	22.36	27.6	34.07	45.01	55.57	68.59	90.61																					
1.5	90	6.9	8.51	9.61	10.47	11.2	11.83	12.38	12.89	13.35	13.78	14.18	14.56	16.44	17.92	23.58	29.02	35.72	47.01	57.86	71.22	93.72																					
1.75	105	7.33	9.01	10.16	11.06	11.82	12.47	13.05	13.58	14.06	14.5	14.92	15.31	17.26	18.8	24.66	30.28	37.18	48.77	59.88	73.52	96.44																					
2	120	7.71	9.47	10.66	11.59	12.38	13.06	13.66	14.21	14.7	15.16	15.6	16	18.01	19.6	25.64	31.41	38.49	50.34	61.68	75.57	98.85																					
2.25	135	8.07	9.89	11.12	12.08	12.9	13.6	14.22	14.78	15.29	15.77	16.21	16.63	18.7	20.33	26.53	32.44	39.68	51.77	63.31	77.43	101.03																					
2.5	150	8.41	10.28	11.55	12.54	13.38	14.1	14.73	15.32	15.84	16.33	16.79	17.21	19.34	21.01	27.36	33.4	40.78	53.09	64.81	79.13	103.02																					
2.75	165	8.72	10.65	11.95	12.97	13.83	14.57	15.22	15.82	16.36	16.85	17.32	17.76	19.94	21.65	28.13	34.29	41.8	54.3	66.2	80.7	104.85																					
3	180	9.02	11	12.33	13.37	14.26	15.01	15.68	16.29	16.84	17.35	17.83	18.27	20.5	22.24	28.85	35.12	42.75	55.44	67.49	82.16	106.55																					
3.25	195	9.3	11.33	12.69	13.76	14.66	15.43	16.11	16.73	17.3	17.82	18.3	18.76	21.03	22.81	29.53	35.9	43.64	56.51	68.7	83.53	108.14																					
3.5	210	9.57	11.64	13.03	14.12	15.04	15.83	16.52	17.16	17.73	18.26	18.76	19.22	21.53	23.34	30.17	36.64	44.49	57.51	69.84	84.81	109.63																					
3.75	225	9.83	11.94	13.36	14.47	15.41	16.21	16.92	17.56	18.14	18.68	19.19	19.66	22.01	23.85	30.78	37.34	45.29	58.46	70.92	86.03	111.04																					
4	240	10.07	12.23	13.67	14.81	15.76	16.57	17.29	17.95	18.54	19.09	19.6	20.08	22.47	24.33	31.37	38.01	46.06	59.37	71.94	87.18	112.38																					
4.25	255	10.31	12.5	13.98	15.13	16.09	16.92	17.65	18.32	18.92	19.48	20	20.48	22.91	24.8	31.92	38.65	46.79	60.23	72.92	88.27	113.65																					
4.5	270	10.54	12.77	14.27	15.43	16.42	17.26	18	18.68	19.29	19.85	20.38	20.87	23.33	25.24	32.46	39.26	47.48	61.06	73.85	89.32	114.85																					
4.75	285	10.76	13.03	14.55	15.73	16.73	17.58	18.33	19.02	19.64	20.21	20.75	21.24	23.73	25.67	32.97	39.85	48.15	61.85	74.74	90.32	116.01																					
5	300	10.98	13.28	14.82	16.02	17.03	17.89	18.66	19.35	19.98	20.56	21.1	21.6	24.12	26.09	33.47	40.41	48.8	62.61	75.6	91.28	117.12																					
5.25	315	11.19	13.52	15.08	16.3	17.32	18.2	18.97	19.67	20.31	20.89	21.44	21.95	24.5	26.49	33.95	40.96	49.42	63.34	76.42	92.2	118.18																					
5.5	330	11.39	13.75	15.34	16.57	17.6	18.49	19.27	19.98	20.63	21.22	21.78	22.29	24.87	26.87	34.41	41.49	50.02	64.04	77.21	93.09	119.2																					
5.75	345	11.59	13.98	15.58	16.83	17.88	18.77	19.56	20.29	20.93	21.53	22.1	22.62	25.22	27.25	34.86	42	50.6	64.73	77.98	93.95	120.18																					
6	360	11.78	14.2	15.82	17.09	18.15	19.05	19.85	20.58	21.24	21.84	22.41	22.94	25.57	27.61	35.29	42.49	51.16	65.39	78.72	94.78	121.13																					
6.25	375	11.96	14.42	16.06	17.33	18.41	19.32	20.13	20.86	21.53	22.14	22.71	23.24	25.9	27.97	35.71	42.97	51.7	66.02	79.44	95.58	122.05																					
6.5	390	12.15	14.63	16.29	17.58	18.66	19.58	20.4	21.14	21.81	22.43	23.01	23.55	26.23	28.31	36.12	43.44	52.23	66.64	80.13	96.35	122.94																					
6.75	405	12.32	14.83	16.51	17.81	18.91	19.84	20.66	21.41	22.09	22.71	23.3	23.84	26.54	28.65	36.52	43.89	52.74	67.24	80.81	97.11	123.8																					
7	420	12.5	15.03	16.73	18.04	19.15	20.09	20.92	21.68	22.36	22.99	23.58	24.13	26.85	28.97	36.91	44.33	53.24	67.83	81.46	97.84	124.64																					
7.25	435	12.67	15.23	16.94	18.27	19.38	20.33	21.17	21.94	22.62	23.26	23.85	24.4	27.16	29.29	37.29	44.76	53.73	68.4	82.1	98.55	125.45																					
7.5	450	12.83	15.42	17.15	18.49	19.61	20.57	21.42	22.19	22.88	23.52	24.12	24.68	27.45	29.61	37.66	45.18	54.2	68.95	82.72	99.24	126.24																					
7.75	465	13	15.61	17.35	18.7	19.84	20.8	21.66	22.43	23.13	23.78	24.38	24.94	27.74	29.91	38.02	45.59	54.67	69.49	83.33	99.91	127.01																					
8	480	13.16	15.79	17.55	18.91	20.06	21.03	21.89	22.68	23.38	24.03	24.64	25.21	28.02	30.21	38.37	45.99	55.12	70.02	83.91	100.57	127.76																					
8.25	495	13.31	15.97	17.74	19.12	20.27	21.26	22.12	22.91	23.62	24.28	24.89	25.46	28.3	30.5	38.72	46.38	55.56	70.53	84.49	101.21	128.49																					
8.5	510	13.47	16.15	17.94	19.32	20.49	21.47	22.35	23.14	23.86	24.52	25.14	25.71	28.57	30.78	39.06	46.76	55.99	71.04	85.05	101.83	129.2																					
8.75	525	13.62	16.33	18.12	19.52	20.69	21.69	22.57	23.37	24.09	24.76	25.38	25.96	28.83	31.06	39.39	47.14	56.41	71.53	85.6	102.44	129.9																					
9	540	13.77	16.5	18.31	19.72	20.9	21.9	22.79	23.59	24.32	24.99	25.62	26.2	29.09	31.34	39.71	47.5	56.83	72.01	86.14	103.04	130.57																					
9.25	555	13.91	16.66	18.49	19.91	21.1	22.11	23	23.81	24.54	25.22	25.85	26.43	29.35	31.61	40.03	47.86	57.23	72.48	86.66	103.62	131.24																					
9.5	570	14.06	16.83	18.67	20.1	21.29	22.31	23.21	24.03	24.76	25.44	26.08	26.67	29.6	31.87	40.34	48.22	57.63	72.94	87.18	104.19	131.89																					
9.75	585	14.2	16.99	18.84	20.28	21.49	22.51	23.42	24.24	24.98	25.66	26.3	26.9	29.84	32.13	40.65	48.56	58.02	73.4	87.68	104.75	132.52																					
10	600	14.34	17.15	19.02	20.46	21.68	22.71	23.62	24.45	25.19	25.88	26.52	27.12	30.09	32.39	40.95	48.9	58.4	73.84	88.18	105.3	133.15																					
10.25	615	14.48	17.31	19.19	20.64	21.87	22.9	23.82	24.65	25.4	26.09	26.74	27.34	30.32	32.64	41.24	49.23	58.77	74.27	88.66	105.84	133.75																					